S-17-51 (E. Academy Street)
Emergency Bridge Replacement over Unnamed Stream
Dillon County, South Carolina

November 29, 2018 SCDOT Project ID.: P038248 Terracon Project No. 7318P119E

#### Prepared for:

South Carolina Department of Transportation Columbia, South Carolina

#### Prepared by:

Terracon Consultants, Inc. Columbia, South Carolina

Offices Nationwide Employee-Owned Established in 1965 terracon.com



November 29, 2018



South Carolina Department of Transportation 955 Park Street, Room 421 Columbia, South Carolina 29201

Attn: Mr. Trapp Harris, P.E.

Geotechnical Design Engineer – Design-Build Section

Re: Geotechnical Data Report

S-17-51 (E. Academy Street) Emergency Bridge Replacement over Unnamed Stream

Dillon County, South Carolina SCDOT Project ID.: P038248

Terracon Project Number: 7318P119E

Dear Mr. Harris:

Terracon Consultants Inc. (Terracon) has completed the geotechnical exploration and testing services for the above referenced project. These services were conducted in general accordance with the SCDOT Request for Subsurface Exploration and Laboratory Testing (SCDOT Project ID: P038248, authorized on November 5, 2018). This geotechnical data report presents the findings of the subsurface exploration and laboratory testing along with an overview of testing activities.

#### 1.0 INTRODUCTION

The South Carolina Department of Transportation (SCDOT) has contracted Terracon to perform subsurface exploration and laboratory testing for the replacement of the S-17-51 (E. Academy Street) bridge over an unnamed stream in Dillon County, SC. The purpose of this work is to develop information relative to subsurface soil and groundwater conditions at the bridge location. No geotechnical recommendations are associated with the requested scope of study.

The following sections of this report contain a summary of the activities for our field exploration and laboratory testing. The logs of the borings/CPT soundings, results of the geophysical testing, the Site Location Map and the Exploration Plan are included in Appendix A. The results of the laboratory testing performed on soil samples obtained from the site during the field exploration are included in Appendix B. Descriptions of the field exploration and laboratory testing are included in their respective appendices.



Terracon Consultants, Inc. 521 Clemson Road Columbia, South Carolina 29229
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S-17-51 (E. Academy Street) RBO Unnamed Stream Dillon County, SC November 29, 2018 Terracon Project No. 7318P119E/Project ID.: P038248



#### 2.0 PROJECT DESCRIPTION

The project site is located at the bridge crossing of S-17-51 (E. Academy Street) and an unnamed stream in Dillon County, South Carolina. It is our understanding that the project will include the replacement of the previously damaged bridge with a new structure on the existing or similar horizontal alignment. The original structure appears to have been supported with deep foundations.

#### 3.0 GEOTECHNICAL TESTING

The geotechnical exploration for this project was performed between November 8 and 12, 2018. The results of our field work and our associated laboratory testing is attached in Appendixes A and B of this report.

#### 3.1 Field Exploration

Our field exploration at the site consisted of the following:

- Two (2) Standard Penetration Test (SPT) Borings (B-1 and B-2)
- Two (2) CPT Soundings (CPT-1 and CPT-2)
- Geophysical testing including one (1) Multi-channel Analysis of Surface Waves test array (MASW-1)

The tests were performed at the approximate locations provided by the SCDOT. A description of our testing methods and graphical logs outlining the soil conditions at each test location are presented in Appendix A. Test locations were established in the field by Terracon and surveyed by Construction Support Services, LLC, after completion. Photographs of the drill rig set up at each boring location are provided in Appendix A.

#### 3.2 Laboratory Testing

The following laboratory tests were performed on the soil samples collected at the site.

- Eight (8) Natural Moisture Content Tests
- Eight (8) Gradation Tests
- Four (4) Atterberg Limits Tests

The scope of the laboratory testing frequency was determined by the SCDOT. The laboratory procedures and results of the laboratory tests are presented in Appendix B.

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#### 4.0 CLOSURE

We appreciate the opportunity to be of service to you on this project. If you have any questions concerning this report or we may be of further service, please contact us.

Sincerely,

**Terracon Consultants, Inc.** 

Phillip A. Morrison, P.E. Geotechnical Department Manager SC Registration No. 17275

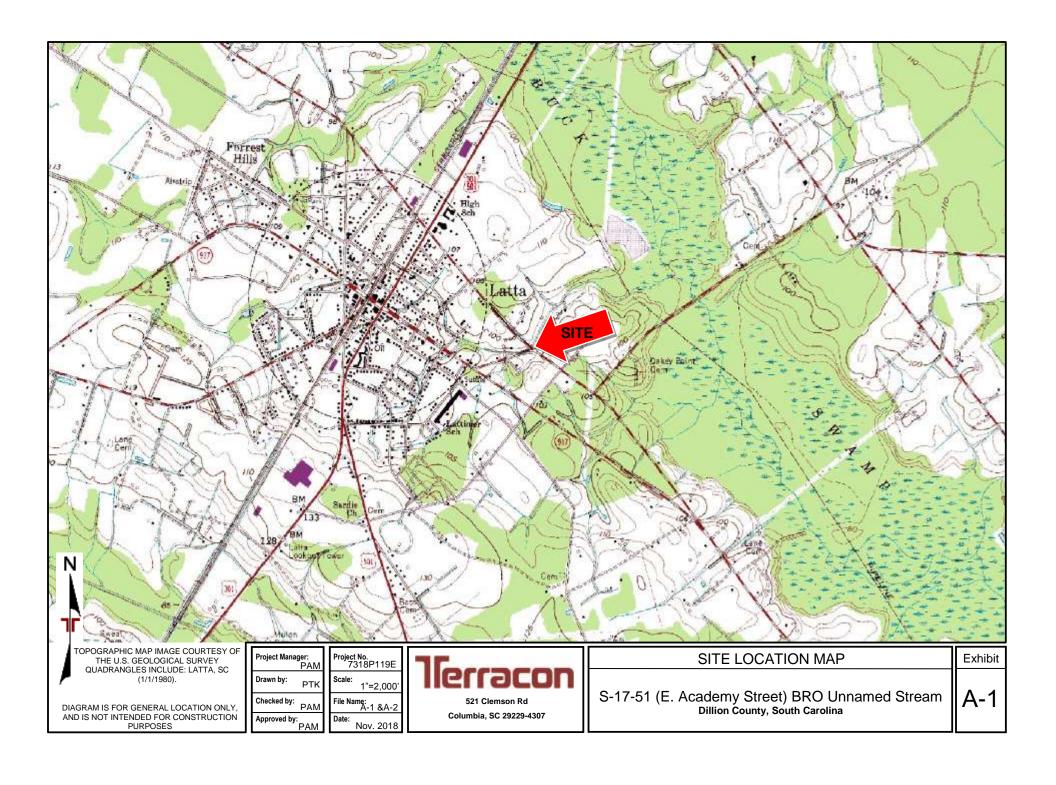
Jonathan N. Ard, P.E. Senior Engineer

Attachments:

Appendix A Appendix B Appendix C

# APPENDIX A FIELD EXPLORATION

Exhibit A-1 – Site Location Map
Exhibits A-2 – Exploration Plan
Exhibit A-3 – Summary of Field Data
Exhibit A-4 – MASW Results
Exhibit A-5 – Field Exploration Description
Exhibit A-6 – Soil Description Terms
Exhibit A-7 – Soil Rock Symbol Log
Exhibit A-8 – Boring Logs
Exhibit A-9 – CPT Logs
Exhibit A-10 – Drill Rig Photograph Log





AERIAL PHOTOGRAPHY PROVIDED BY MICROSOFT BING MAPS

DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES

Project Manager: PAM Scale: AS SHOWN Drawn by: PTK Checked by: PAM File Name: A-1 &A-2 Approved by: PAM Date:

Nov. 2018

521 Clemson Rd Columbia, SC 29229-4307

S-17-51 (E. Academy Street) BRO Unnamed Stream Dillion County, South Carolina

A-2

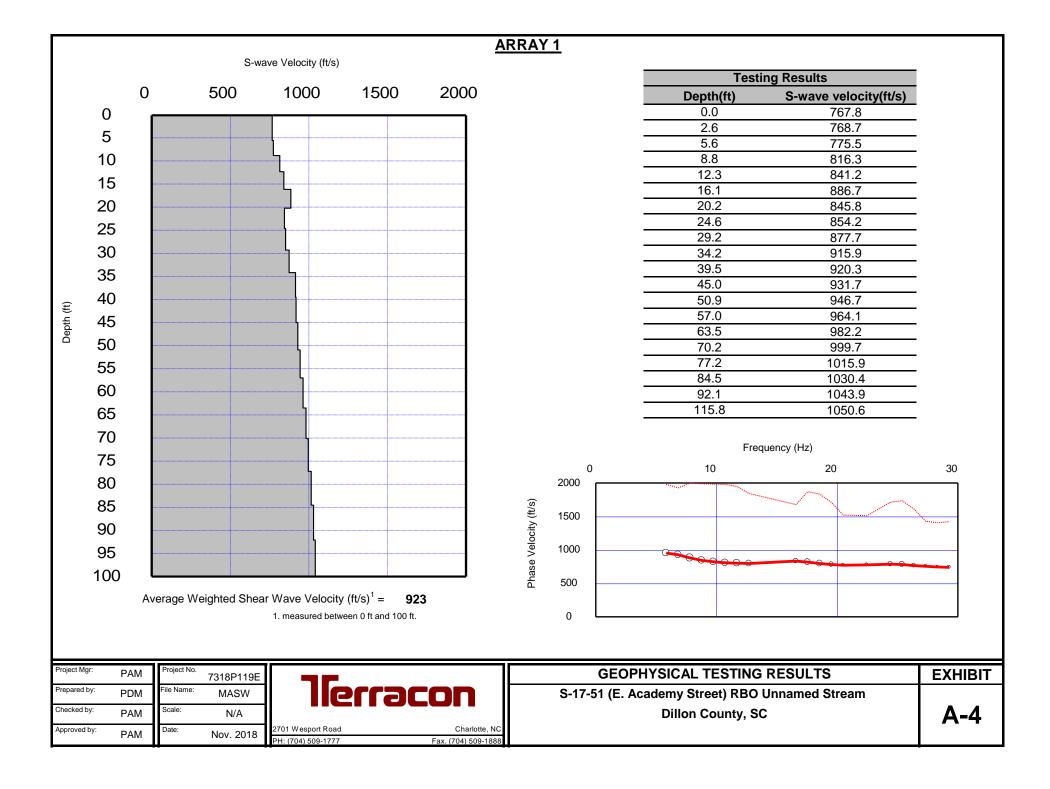
S-17-51 (E. Academy Street) RBO Unnamed Stream Dillon County, SC November 29, 2018 Terracon Project No. 7318P119E/Project ID.: P038248



## **Summary of Field Data**

Test No.	Ground Elevation (ft)	Test Depth (ft.)	Northing	Easting	Latitude	Longitude
B-1	96.90	100	913773.92	2476805.03	N34.334809	W79.420399
B-2	96.91	100	913787.52	2476756.26	N34.334848	W79.420560
CPT-1	96.33	37.8	913766.03	2476785.14	N34.334788	W79.420466
CPT-2	96.67	33.1	913808.48	2476770.84	N34.334905	W79.420511
MASW-11	97.81	N.A.	913896.87	2476629.88	N34.335154	W79.420973

<sup>1.</sup> Approximate center of array



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#### FIELD EXPLORATION DESCRIPTION

#### Overview

The general testing locations were provided by the SCDOT and located in the field by Terracon by taking measurements from existing structures. The borings were surveyed by Construction Support Services, LLC after testing and drilling was complete. The locations are shown on the appended Exploration Plan.

A field log of each test location was prepared by our field engineer. The final test logs included with this report represent the engineer's description of the encountered conditions modified as necessary based on laboratory test results of the individual samples.

#### Soil Test Borings (STB)

All boring and sampling operations were conducted in general accordance with the following procedures:

- SCDOT Geotechnical Design Manual 2010
- ASTM D5783, "Standard Guide for Use of Direct Rotary Drilling with Water-Based Drilling Fluid for Geo-environmental Exploration"
- ASTM D1586 "Test Method for Penetration Test and Split-Barrel Sampling of Soils"
- ASTM D4220 "Standard Practices for Preserving and Transporting Soil"

Each boring was advanced using rotary wash drilling techniques to the planned termination depths.

#### Cone Penetration Test Soundings (CPT)

Cone Penetration Test soundings were conducted in general accordance with ASTM D5778 "Standard Test Method for Performing Electronic Friction Cone and Piezocone Penetration Testing of Soils.

The sampling program is summarized in the following table:

Test ID	Total Depth	Interval of Continuous Sampling
B-1	100 feet or refusal and 20 feet of coring	0 to 10 feet
B-2	100 feet or refusal and 20 feet of coring	0 to 10 feet
CPT-1	50 feet or refusal	N/A
CPT-2	50 feet or refusal	N/A

Soil samples were obtained with a standard 1.4-inch I.D., 2-inch O.D., split-barrel sampler, also known as a standard split-spoon. The sampler is advanced into the soil a total of 18 to 24 inches by striking the drill rod using a 140-pound automatic hammer falling 30 inches. The number of

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blows required to advance the sampler for each of three to four, 6-inch increments is recorded. The sum of the number of blows for the second and third increments is called the "Standard Penetration Value", or N-value (N<sub>meas</sub>, blows per foot). The N-value, when properly evaluated, is an index to the soil strength.

Soil Classification provides a general guide to the engineering properties of various soil types and enables the engineer to apply his experience to current situations. In our exploration, samples obtained during drilling operations are examined and visually classified by a geotechnical engineer using the procedures outlined in ASTM D2487 - Standard Classification of Soils for Engineering Purposes (Unified Soil Classification System). Laboratory testing was also performed on select split-spoon samples to evaluate index properties for further classification. The soils are described according to color, texture, and relative density or consistency (based on standard penetration resistance). The designations shown on the logs are described on Exhibit A-6.

Consistent with SCDOT GDM, the borings were drilled using mud rotary drilling techniques. As the drilling method introduces water into the borehole, time-of-drilling water levels could not be recorded. As noted on the boring log, the water levels of the borings were recorded 1 day after the start of drilling activities. These water levels are indicated on the boring logs. At the conclusion of the work, the boreholes were backfilled with clean sand and the borings were capped with coldpatch asphalt.

#### **Seismic Surface Wave Testing**

Multi-Channel Analysis of Surface Waves (MASW testing) was performed to determine the shear wave velocity profile of the layered soil system. At the test location, both MASW readings (active) and Microtremor Array Measurement (MAM) readings (passive) were recorded. The MASW test was conducted using the 24-channel Geometrics Geode seismographs and 4.5-Hz geophones with a linear geometry at an interval of 10-ft. Surface waves were generated by a 20-pound sledgehammer striking a polyethylene plate at four locations. MAM testing was performed along the same survey line.

MASW (Active) Testing - Multi-Channel Analysis of Surface Waves (MASW) is a seismic method that uses the dispersive characteristics of Rayleigh-type surface waves to determine the variation of the shear-wave velocity of layered soils with depth.

MAM (Passive) Testing - Microtremor Array Measurement (MAM) "for lower frequency surface waves (passive waves) arising from microtremors and/or urban (traffic) noise and recorded them using a linear or two-dimensional (triangle, circle, semicircle, and "shapes") array of geophones (Zywicki and Rix, 1999; Lie et al., 2000). Multiple noise records are required for analysis. The data filters out the Rayleigh waves through a technique called spatial auto-correction (SPAC). This allows the development of a dispersion curve that is defined as the lower envelope of the measured energy peaks. MAM testing results in lower peak energy selections than the active testing described above.

#### SOIL DESCRIPTION TERMS

#### **Relative Density/Consistency Terms**

Relative Density <sup>1</sup>			Consistency <sup>2</sup>		
				Unconfined	SPT Blow
Descriptive Term	Relative Density	SPT Blow Count	Descriptive Term	Compression	Count
				Strength (q <sub>u</sub> ) (tsf)	
Very Loose	0 to 15%	4 and less	Very Soft	0.25 and less	2 and less
Loose	16 to 35%	5 to 10	Soft	0.26 to 0.50	3 to 4
Medium Dense	36 to 65%	11 to 30	Firm	0.51 to 1.00	5 to 8
Dense	66 to 85%	31 to 50	Stiff	1.01 to 2.00	9 to 15
Very Dense	86 to 100%	51 and more	Very Stiff	2.01 to 4.00	16 to 30
			Hard	4.01 and more	31 and more

#### **Moisture Condition**

Descriptive Term Criteria

Dry Absence of moisture, dusty, dry to the touch

Moist Damp but no visible water

Wet Visible free water, usually in coarse-grained soils below the water table

#### Color

Describe the sample color while sample is still moist.

#### Angularity<sup>1</sup>

<u>Descriptive Term</u> <u>Criteria</u>

Angular Particles have sharp edges and relatively plane sides with unpolished surfaces.

Subangular Particles are similar to angular description but have rounded edges.

Subrounded Particles have nearly plane sides but have well-rounded corners and edges.

Rounded Particles have smoothly curved sides and no edges.

#### HCI Reaction<sup>3</sup>

Descriptive Term Criteria

None Reactive No visible reaction

Weakly Reactive Some reaction, with bubbles forming slowly
Strongly Reactive Violent reaction, with bubbles forming immediately

#### Cementation<sup>3</sup>

Descriptive Term Criteria

Weakly Cemented Crumbles or breaks with handling or little finger pressure
Moderately Cemented Crumbles or breaks with considerable finger pressure
Strongly Cemented Will not crumble or break with finger pressure

#### Particle-Size Range<sup>1</sup>

Gravel	Diameter, mm	Sieve Size	<u>Sand</u>	Diameter, mm	Sieve Size
Fine	4.76 to 19.1	#4 to ¾ inch	Fine	0.074 to 0.42	#200 to #40
Coarse	19.1 to 76.2	34 inch to 3 inch	Medium	0.42 to 2.00	#40 to #10
			Coarse	4 00 to 4 76	#10 to #4

#### Primary Soil Type<sup>1, 2</sup>

The primary soil type will be shown in all capital letters.

#### **USCS Soil Designation**

Indicate USCS soil designation as defined in ASTM D-2487 and D-2488

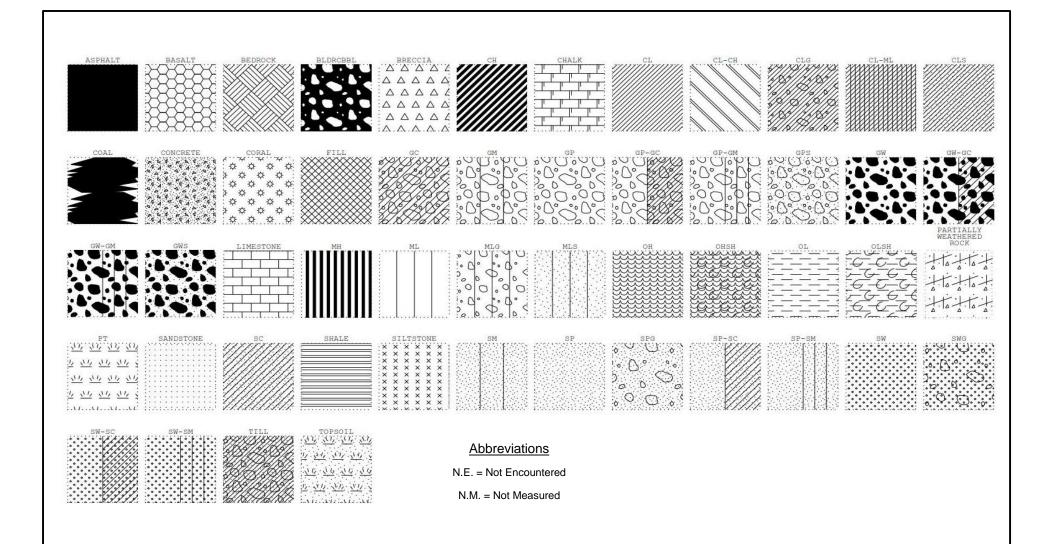
#### **AASHTO Soil Designation**

Indicate AASHTO soil designation as defined in AASHTO M-145 and ASTM D-3282

<sup>&</sup>lt;sup>1</sup> Applies to coarse-grained soils (major portion retained on No. 200 sieve)

<sup>&</sup>lt;sup>2</sup> Applies to fine-grained soils (major portion passing No. 200 sieve)

<sup>&</sup>lt;sup>3</sup>Use as required



Project Manager:	Project No.
KJZ	7318P119B
Drawn by:	Scale:
KJZ	N.T.S.
Checked by:	File Name:
KJZ	File Name: Soil — Rock — Log
Approved by:	Date:



PH. (803) 741-9000

JULY 2016

FAX. (803) 741-9900

SOIL / ROCK / LOG SYMBOL LEGEND



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SAMPLER TYPE
NQ - Rock Core, 1-7/8"
e CU - Cuttings
CT - Continuous Tube SS - Split Spoon UD - Undisturbed Sample AWG - Rock Core, 1-1/8"

HSA - Hollow Stem Auger CFA - Continuous Flight Augers DC - Driving Casing

DRILLING METHOD er RW - Rotary Wash Augers RC - Rock Core

SC\_DOT 7318P119E



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#### **CPT LOG NO. CPT-1** Page 1 of 1 PROJECT: S-17-51 (E. Academy Street) BRO **CLIENT: SCDOT** TEST LOCATION: See Exhibit A-2 **Unnamed Stream** Columbia, South Carolina **SITE:** Various Counties Latitude: 34.334788° Dillion County, South Carolina Longitude: -79.420465° **Hydrostatic Pressure** Material 12 18 0.16 0.32 0.48 0.64 Pore Pressure, u<sub>2</sub> Description Tip Resistance, q<sub>t</sub> Sleeve Friction, f<sub>s</sub> Friction Ratio, F, Depth Depth Normalized CPT (tsf) (ft) (tsf) (tsf) (%) Soil Behavior Type 120 180 3.2 12345678 0.5 1.0 1.5 2.0 2.5 3.0 3.5 20 25 30 30 35 CPT Terminated at 37.8 Feet Sensitive, fine grained Organic soils - clay See Exhibit A-5 for description of field procedures. CPT sensor calibration reports available upon request. 2 Organic Suins - Clay 3 Clay - silty clay to clay 4 Silt mixtures - clayey silt to silty clay 5 Sand mixtures - silty sand to sandy silt 6 Sands - clean sand to silty sand 7 Gravelly sand to dense sand See Appendix C for explanation of symbols and abbreviations. Very stiff sand to clayey sand Very stiff fine grained Probe no. 4815 with net area ratio of 0.88 WATER LEVEL OBSERVATION CPT Started: 11/14/2018 CPT Completed: 11/14/2018 U2 pore pressure transducer location Manufactured by Geotech A.B.; calibrated 2/20/2017 Rig: Pagani TG73-200 Operator: BR Tip and sleeve areas of 10 cm<sup>2</sup> and 150 cm<sup>2</sup> (used in normalizations and correlations) Ring friction reducer with O.D. of 1.875 in Project No.: 7318P119E Exhibit: A-1 see Appendix C) Columbia, SC

.GPJ

7318P119E S-51 LAB DATA

SEPARATED FROM ORIGINAL REPORT

#### **CPT LOG NO. CPT-2** Page 1 of 1 PROJECT: S-17-51 (E. Academy Street) BRO **CLIENT: SCDOT TEST LOCATION:** See Exhibit A-2 **Unnamed Stream** Columbia, South Carolina **SITE:** Various Counties Latitude: 34.334905° Dillion County, South Carolina Longitude: -79.420511° **Hydrostatic Pressure** Material 12 18 24 0.16 0.32 0.48 0.64 Pore Pressure, u<sub>2</sub> Description Tip Resistance, q<sub>t</sub> Sleeve Friction, f<sub>s</sub> Friction Ratio, F, Depth Depth Normalized CPT (tsf) (ft) (tsf) (tsf) (%) Soil Behavior Type 120 180 3.2 4.8 6.4 12345678 0.5 1.0 1.5 2.0 2.5 3.0 3.5 15 20 25 30 CPT Terminated at 33.1 Feet 35 Sensitive, fine grained Organic soils - clay CPT sensor calibration reports available upon request. See Exhibit A-5 for description of field procedures. 2 Organic Suins - Clay 3 Clay - silty clay to clay 4 Silt mixtures - clayey silt to silty clay 5 Sand mixtures - silty sand to sandy silt 6 Sands - clean sand to silty sand 7 Gravelly sand to dense sand See Appendix C for explanation of symbols and abbreviations. Very stiff sand to clayey sand Very stiff fine grained Probe no. 4815 with net area ratio of 0.88 WATER LEVEL OBSERVATION CPT Started: 11/14/2018 CPT Completed: 11/14/2018 U2 pore pressure transducer location Manufactured by Geotech A.B.; calibrated 2/20/2017 ✓ 13 ft measured water depth Rig: Pagani TG73-200 Operator: BR Tip and sleeve areas of 10 cm<sup>2</sup> and 150 cm<sup>2</sup> (used in normalizations and correlations; Ring friction reducer with O.D. of 1.875 in Project No.: 7318P119E Exhibit: A-2 Columbia, SC see Appendix C)

DATATEMPLATE

GPJ TERRACON

7318P119E S-51 LAB DATA

SEPARATED FROM ORIGINAL REPORT.

SEE CPT LOG NO. CPT-1 FOR DETAILED TEST RESULTS

PROJECT: S-17-51 (E. Academy Street) BRO CLIENT: SCDOT

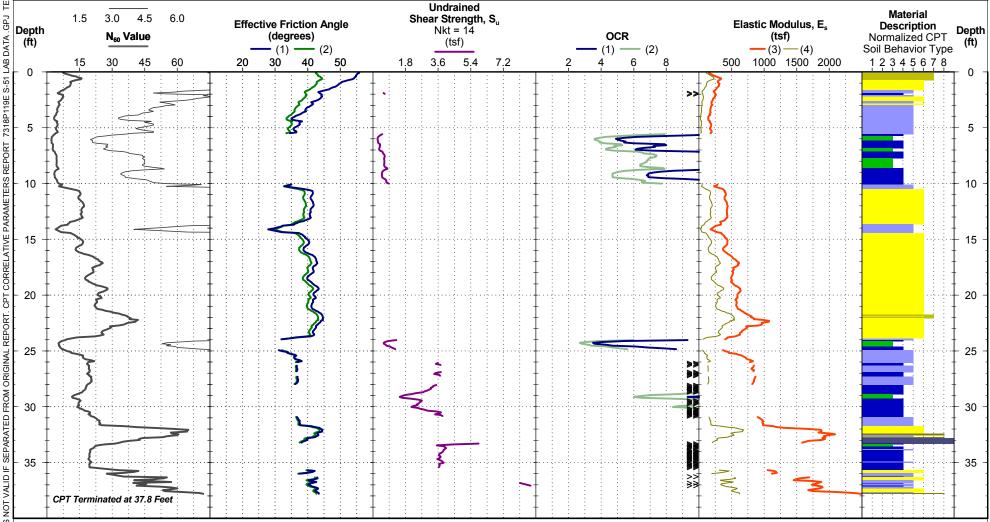
Unnamed Stream

Columbia, South Carolina

**TEST LOCATION:** See Exhibit A-2

SITE: Various Counties
Dillion County, South Carolina

Latitude: 34.334788° Longitude: -79.420465°



Tip resistance, sleeve resistance, porewater pressure, and tilt angle are measured. Other parameters presented are derived from interpretations of the measured data, based upon published correlations, but do not necessarily represent actual values that would be derived from direct testing. Appendix C provides the formulas used for these correlations and presents estimates of the relative reliability associated with the correlated parameters.

WATER LEVEL OBSERVATION

▼ 14 ft measured water depth (used in normalizations and correlations; see Appendix C) Probe no. 4815 with net area ratio of 0.88 Manufactured by Geotech A.B.; calibrated 2/20/2017 Tip and sleeve areas of 10 cm<sup>2</sup> and 150 cm<sup>2</sup> Ring friction reducer with O.D. of 1.875 in



CPT Started: 11/14/2018	CPT Completed: 11/14/2018
Rig: Pagani TG73-200	Operator: BR
Project No.: 7318P119E	Exhibit: A-1

## **CPT CORRELATIVE PARAMETER LOG NO. CPT-2**

Page 1 of 1

SEE CPT LOG NO. CPT-2 FOR DETAILED TEST RESULTS

PROJECT: S-17-51 (E. Academy Street) BRO CLIENT: SCDOT

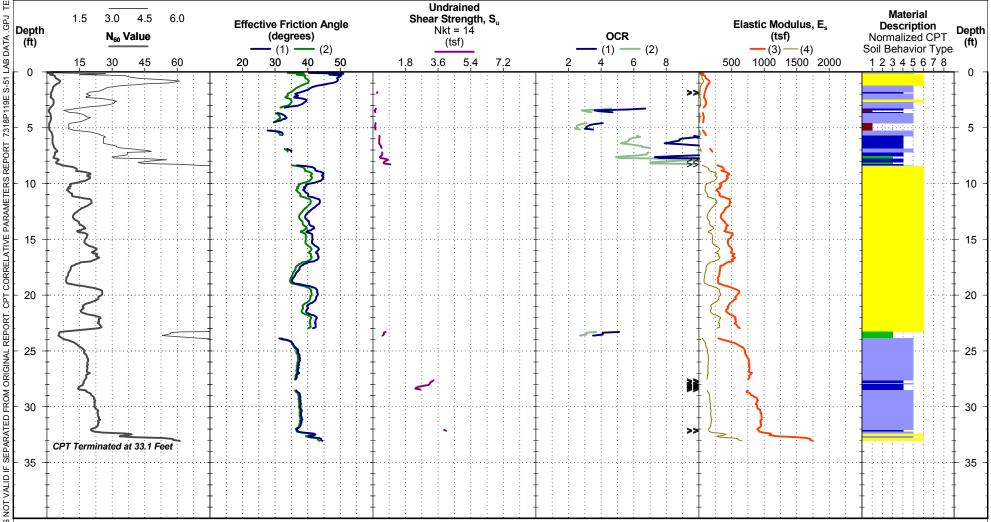
Unnamed Stream

Columbia, South Carolina

**TEST LOCATION:** See Exhibit A-2

SITE: Various Counties
Dillion County, South Carolina

Latitude: 34.334905° Longitude: -79.420511°



Tip resistance, sleeve resistance, porewater pressure, and tilt angle are measured. Other parameters presented are derived from interpretations of the measured data, based upon published correlations, but do not necessarily represent actual values that would be derived from direct testing. Appendix C provides the formulas used for these correlations and presents estimates of the relative reliability associated with the correlated parameters.

WATER LEVEL OBSERVATION

▼ 13 ft measured water depth (used in normalizations and correlations see Appendix C) Probe no. 4815 with net area ratio of 0.88 Manufactured by Geotech A.B.; calibrated 2/20/2017 Tip and sleeve areas of 10 cm<sup>2</sup> and 150 cm<sup>2</sup> Ring friction reducer with O.D. of 1.875 in



CPT Started: 11/14/2018	CPT Completed: 11/14/2018
Rig: Pagani TG73-200	Operator: BR
Project No.: 7318P119E	Exhibit: A-2





Drill rig on B-1



Drill rig on B-2





CPT rig on CPT-1



CPT rig on CPT-2

# APPENDIX B LABORATORY TESTING

Exhibit B-1 – Laboratory Testing Description Exhibit B-2 – Summary of Laboratory Data Laboratory Data Sheets

S-17-51 (E. Academy Street) RBO Unnamed Stream Dillon County, SC November 29, 2018 Terracon Project No. 7318P119E/Project ID.: P038248



#### LABORATORY TESTING DESCRIPTION

The samples collected during the field exploration were taken to our laboratory for additional testing. The laboratory testing program was developed by the SCDOT. Using the provided testing program, the laboratory tests were conducted on selected soil samples from the borings. The test results are presented in this appendix.

The laboratory test results were used to confirm the soil descriptions presented on the boring logs in Appendix A. Laboratory tests were performed in general accordance with the applicable ASTM, AASHTO, SCDOT or other accepted standards.

Selected soil samples obtained from the site were tested for the following engineering properties:

Particle-Size Distribution (Gradation) of Soils

Liquid Limit, Plastic Limit and Plasticity Index of Soils

Determination of Moisture Content of Soils

**ASTM D6913** 

AASHTO T89/90/(ASTM D4318)

**AASHTO T265/(ASTM D2216)** 

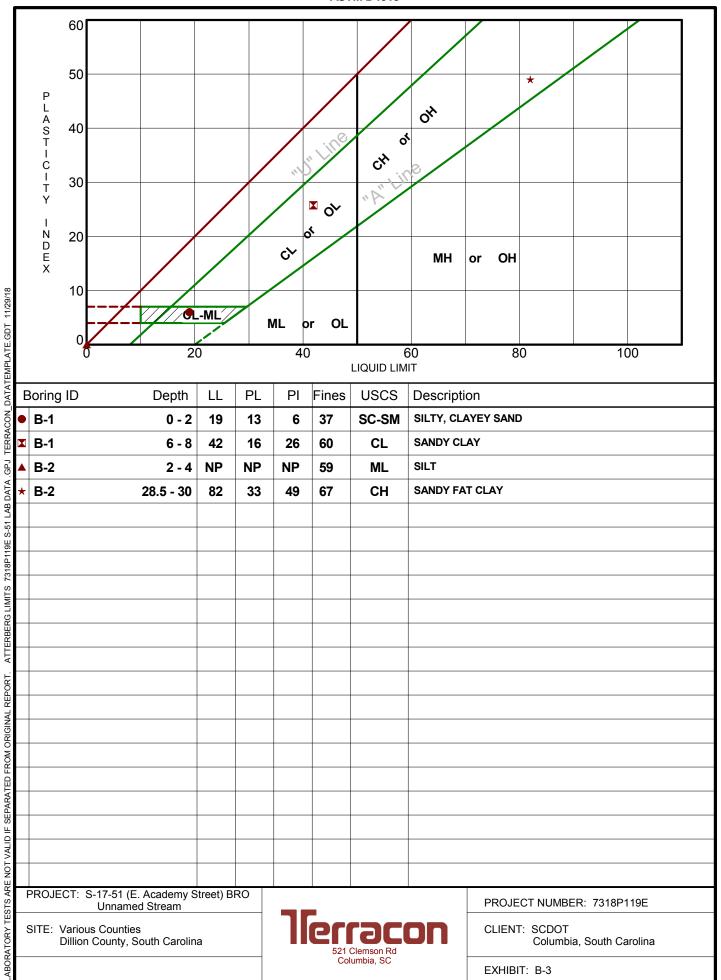
**Summary of Laboratory Results** 

			J	UI LADUIA	,			Sheet 1 of		
BORING ID	Depth (Ft.)	% Gravel	% Sand	% Fines	Liquid Limit	Plastic Limit	Plasticity Index	Water Content (%		
B-1	0 - 2	1.0	61.8	37.2	19	13	6	11		
B-1	6 - 8	0.0	39.8	60.2	42	16	26	25		
B-1	18.5 - 20	1.3	93.1	5.6				21		
B-1	38.5 - 40	0.7	96.5	2.8				19		
B-2	2 - 4	0.0	40.5	59.5	NP	NP	NP	19		
B-2	13.5 - 15	1.9	92.1	6.0				18		
B-2	28.5 - 30	1.3	31.5	67.1	82	33	49	59		
B-2	43.5 - 45	3.2	79.6	17.2				34		
PROJECT: S	Unnamed Stre	demy Street) BR0 eam		stac	'OP		 MBER: 7318P1 OT	19E		
Dillion	County, South	Carolina		521 Clemson Rd Columbia, SC	ا الا.	CLIENT: SCDOT Columbia, South Carolina				
				Columbia, GO		EXHIBIT: B-2				



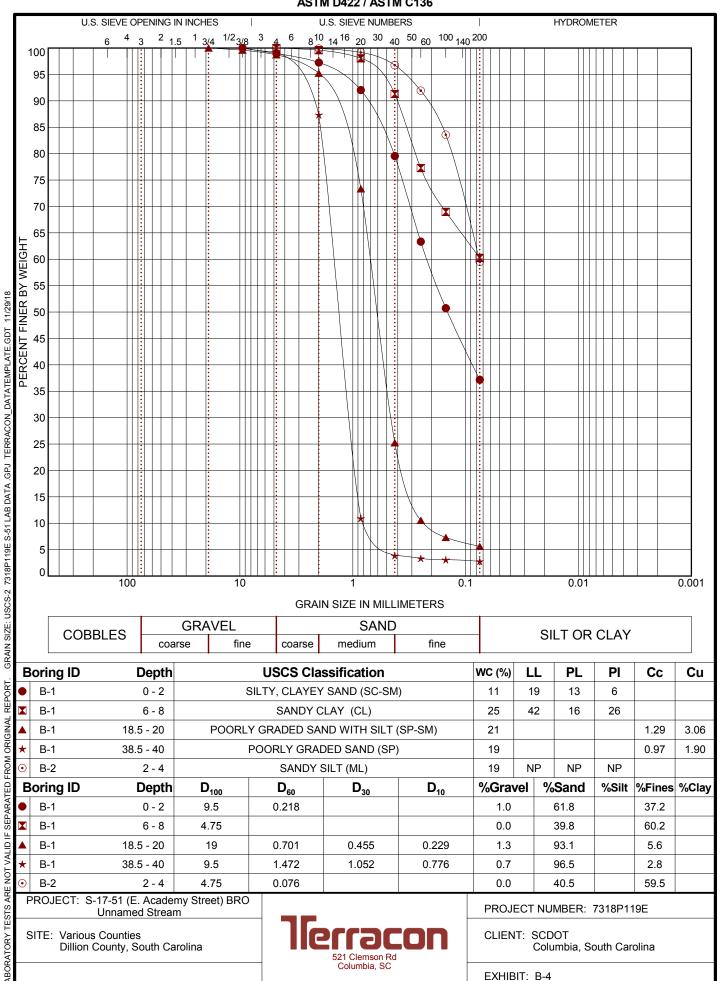
### ATTERBERG LIMITS RESULTS

**ASTM D4318** 



#### GRAIN SIZE DISTRIBUTION

#### **ASTM D422 / ASTM C136**



# APPENDIX C SUPPORTING DOCUMENTS

Exhibit C-1 – CPT General Notes

Exhibit C-2 – Rig Calibration Documentation

#### **CPT GENERAL NOTES**

#### **DESCRIPTION OF MEASUREMENTS AND CALIBRATIONS**

#### To be reported per ASTM D5778:

Uncorrected Tip Resistance, q<sub>c</sub> Measured force acting on the cone divided by the cone's projected area

Corrected Tip Resistance, q<sub>t</sub>
Cone resistance corrected for porewater and net area ratio effects  $q_t = q_c + U2(1 - a)$ 

Where a is the net area ratio, a lab calibration of the cone typically between 0.70 and 0.85

Pore Pressure, U1/U2

Pore pressure generated during penetration U1 - sensor on the face of the cone U2 - sensor on the shoulder (more common)

Sleeve Friction, fs Frictional force acting on the sleeve

divided by its surface area Normalized Friction Ratio, FR

The ratio as a percentage of fs to q, accounting for overburden pressure To be reported per ASTM D7400, if collected:

Shear Wave Velocity, Vs

Measured in a Seismic CPT and provides direct measure of soil stiffness

#### **DESCRIPTION OF GEOTECHNICAL CORRELATIONS**

Normalized Tip Resistance, Q,  $Q_t = (q_t - \sigma_{V0})/\sigma'_{V0}$ Over Consolidation Ratio, OCR  $OCR(1) = 0.25(Q_i)$ OCR (2) =  $0.33(Q_t)$ 

Undrained Shear Strength, Su

Su =  $Q_t \times \sigma'_{VO}/N_{kt}$   $N_{kt}$  is a geographical factor (shown on Su plot)

Sensitivy, St

 $St = (q_t - \sigma_{V0}/N_{kt}) \times (1/fs)$ 

Effective Friction Angle, ¢  $\phi'(1) = \tan^{-1}(0.373[\log(q_1/\sigma'_{V0}) + 0.29])$ 

 $\phi'(2) = 17.6 + 11[\log(Q_i)]$ 

Unit Weight

 $UW = (0.27[log(FR)]+0.36[log(q/atm)]+1.236) \times UW_{w}$  $\sigma_{vo}$  is taken as the incremental sum of the unit weights

Small Strain Shear Modulus, G<sub>0</sub>

 $G_0 (1) = \rho V s^2$   $G_0 (2) = 0.015 \times 10^{(0.55 lc + 1.68)} (q_t - \sigma_{V0})$ 

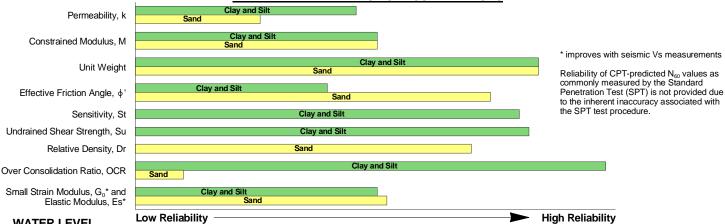
```
Soil Behavior Type Index, Ic
    Ic = [(3.47 - log(Q_t)^2 + (log(FR) + 1.22)^2]^{0.5}
SPT N<sub>60</sub>
N<sub>60</sub> = (q<sub>t</sub>/atm) / 10^{(1.1268 - 0.2817 \, k)}
Elastic Modulus, Es (assumes q/q_{ultimate} ~ 0.3, i.e. FS = 3) Es (1) = 2.6\PsiG_0 where \Psi = 0.56 - 0.33logQ_{t,clean\ sand}
    Es (3) = 0.015 \times 10^{(0.55lc + 1.68)} (q_t - \sigma_{V0})
    Es(4) = 2.5a
Constrained Modulus, M
     M = \alpha_M(q_t - \sigma_{VO})
     For Ic > 2.2 (fine-grained soils)
       \alpha_{\rm M} = Q_{\rm t} with maximum of 14
     For Ic < 2.2 (coarse-grained soils)
        \alpha_{\rm M} = 0.0188 \times 10^{(0)}
Hydraulic Conductivity, k
    For 1.0 < lc < 3.27 k = 10^{(0.952 - 3.04/c)}
For 3.27 < lc < 4.0 k = 10^{(4.52 - 1.37/c)}
```

#### REPORTED PARAMETERS

CPT logs as provided, at a minimum, report the data as required by ASTM D5778 and ASTM D7400 (if applicable). This minimum data include tip resistance, sleeve resistance, and porewater pressure. Other correlated parameters may also be provided. These other correlated parameters are interpretations of the measured data based upon published and reliable references, but they do not necessarily represent the actual values that would be derived from direct testing to determine the various parameters. The following chart illustrates estimates of reliability associated with correlated parameters based upon the literature referenced below.

Relative Density, Dr Dr =  $(Q_1/350)^{0.5}$  x 100

#### RELATIVE RELIABILITY OF CPT CORRELATIONS



#### **WATER LEVEL**

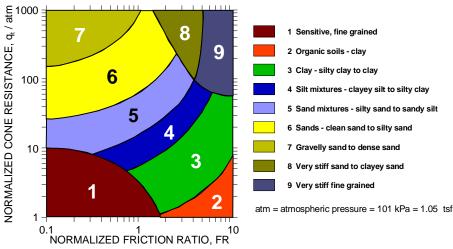
The groundwater level at the CPT location is used to normalize the measurements for vertical overburden pressures and as a result influences the normalized soil behavior type classification and correlated soil parameters. The water level may either be "measured" or "estimated." Measured - Depth to water directly measured in the field

Estimated - Depth to water interpolated by the practitioner using pore pressure measurements in coarse grained soils and known site conditions While groundwater levels displayed as "measured" more accurately represent site conditions at the time of testing than those "estimated," in either case the groundwater should be further defined prior to construction as groundwater level variations will occur over time.

#### **CONE PENETRATION SOIL BEHAVIOR TYPE**

The estimated stratigraphic profiles included in the CPT logs are based on relationships between corrected tip resistance (q<sub>t</sub>), friction resistance (fs), and porewater pressure (U2). The normalized friction ratio (FR) is used to classify the soil behavior

Typically, silts and clays have high FR values and generate large excess penetration porewater pressures; sands have lower FRs and do not generate excess penetration porewater pressures. Negative pore pressure measurements are indicative of fissured fine-grained material. The adjacent graph (Robertson et al.) presents the soil behavior type correlation used for the logs. This normalized SBT chart, generally considered the most reliable, does not use pore pressure to determine SBT due to its lack of repeatability in onshore CPTs.



#### REFERENCES

Kulhawy, F.H., Mayne, P.W., (1997). "Manual on Estimating Soil Properties for Foundation Design," Electric Power Research Institute, Palo Alto, CA. Mayne, P.W., (2013). "Geotechnical Site Exploration in the Year 2013," Georgia Institue of Technology, Atlanta, GA. Robertson, P.K., Cabal, K.L. (2012). "Guide to Cone Penetration Testing for Geotechnical Engineering," Signal Hill, CA. Schmertmann, J.H., (1970). "Static Cone to Compute Static Settlement over Sand," Journal of the Soil Mechanics and Foundations Division, 96(SM3), 1011-1043.



# DRILL RIG SPT HAMMER ENERGY CALIBRATION REPORT

Drill Rig Model CME-45C SN 406484
Terracon Drill Rig No. 543
Gaston Solar Facility

May 29, 2018 Project No. 73185061

#### Prepared for:

Terracon Consultants, Inc. Columbia, SC

#### Prepared by:

Terracon Consultants, Inc. North Charleston, South Carolina



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#### 1.0 PROJECT INFORMATION

ITEM	DESCRIPTION
Drill Rig Identification	CME-45C, SN 406484 (see photograph on cover page)
Drill Rig Owner	Terracon
Drill Rig Operator	Spencer Blair
Testing Date	May 24, 2018
Testing Location	Gaston Solar Facility
Terracon Project Number	73185061
Boring Identification	SB-2
Energy Measurement Depths	30.0 ft.; 35.0 ft.; 40.0 ft.; 45.0 ft.; 50.0 ft.
Hammer Type	Automatic
Boring Method	Hollow Stem Auger
Drill Rods	nAWJ n1¾" outside diameter n3/16" wall thickness
SPT Calibration Testing Equipment	n2 foot AWJ rod instrumented w/ 2 strain gauges and 2 accelerometers nModel PAX Pile Driving Analyzer™ (PDA)
SPT Calibration Personnel	R. Wakeford & Z. McIntosh

#### **TEST RESULTS** 2.0

Table 1: SPT Hammer Energy Calibration Testing Summary.

Boring	Start Depth <sup>1</sup> (ft)	Rod Length <sup>2</sup> (ft)	Rod Sections <sup>3</sup>		Measured Blow Counts (blows/6 inches)				SPT N <sub>meas</sub>	Soil	
J			2 ft	5 ft	10 ft	1 <sup>st</sup> Inc.	2 <sup>nd</sup> Inc.	3 <sup>rd</sup> Inc.	4 <sup>th</sup> Inc.	(bpf)	Type⁴
	28.5	30.0	0	6	0	9	12	15	-	27	SM
SB-2	33.5	35.0	0	7	0	6	5	9	-	14	SM
	38.5	40.0	0	8	0	9	14	19	-	33	SM
	43.5	45.0	0	9	0	8	12	12	-	24	SM
	48.5	50.0	0	10	0	9	16	15	-	31	SM

- 1. Depth from existing ground surface to start of SPT
- 2. Total rod length from instrumentation to bottom of sampler
- 3. Two foot section is instrumented and is located at top of drill rods
- 4. Soil type provided by Terracon personnel.



**Table 2:** Energy Measurement and Analysis Summary.

Boring	Start Depth <sup>1</sup> (ft)	SPT	No. of Blows <sup>2</sup>		EMX <sup>3</sup>	ETR <sup>3</sup> (%)			
		N <sub>m</sub> (bpf)		Max.	Min.	Ave.	Std. Dev.	Ave.	Std. Dev.
	28.5	27	27	0.37	0.31	0.34	0.016	97.9	4.464
SB-2	33.5	14	14	0.36	0.33	0.34	0.008	98.5	2.171
	38.5	33	33	0.35	0.34	0.34	0.003	97.1	0.776
	43.5	24	24	0.34	0.33	0.33	0.004	95.4	1.005
	48.5	31	31	0.36	0.33	0.34	0.006	96.7	1.763
		Average:		0.35	0.33	0.34	0.007	97.1	2.036

<sup>1.</sup> Boring ID and depth from existing ground surface to start of SPT

**Table 3:** Hammer Blow Rate Summary.

Boring	Start Depth <sup>1</sup>	SPT	No.	BPM <sup>3</sup>				
	(ft)	N <sub>meas</sub> (bpf)	of - Blows²	Max.	Min.	Ave.	Std. Dev.	
	28.5	27	27	55.4	54.8	55.1	0.183	
SB-2	33.5	14	14	55.4	55.0	55.2	0.107	
	38.5	33	33	55.0	54.5	54.7	0.123	
	43.5	24	24	46.4	45.6	46.0	0.257	
	48.5	31	31	55.2	54.7	55.0	0.128	
		Average:		53.5	52.9	53.2	0.159	

<sup>1.</sup> Boring ID and depth from existing ground surface to start of SPT.

#### 3.0 CONCLUSIONS

#### 3.1 Energy Transfer Ratio (ETR) and Hammer Efficiency Correction (CE)

Based on our testing and subsequent analysis, drill rig CME-45C (No. 543) has an **ETR** of **97.1%**  $\pm$  **2.04%.** Based on this ETR, the hammer efficiency correction ( $C_E$ ) is **1.62**.

<sup>2.</sup> Number of blows used in energy calibration analysis; limited to measurements recorded during the second and third 6-inch sampling intervals at each depth or during the first increment if refusal were encountered

<sup>3.</sup> EMX = Maximum Transferred Energy, ETR = Energy Transfer Ratio.

Number of blows used in energy calibration analysis. Limited to measurements recorded during the second and third 6-inch sampling intervals at each depth or during the 1st increment if refusal conditions were encountered.

<sup>3.</sup> BPM = Blows per minute

**Drill Rig SPT Hammer Energy Calibration Report** CME-45C (SN 406484) **Columbia**, SC

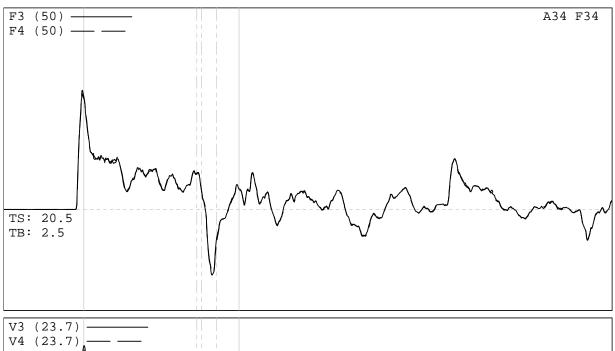
May 29, 2018 Terracon Project No. 73185061

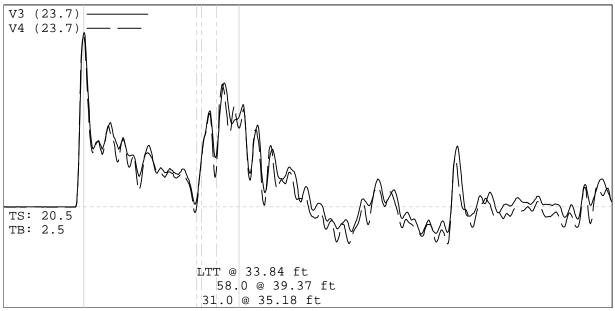


## **Exhibit A-1** Representative Blow

#### **GASTON 2 SOLAR FACILITY**

SPT CAL 38.5-40





#### **Project Information**

PROJECT: GASTON 2 SOLAR FACILITY

PILE NAME: SPT CAL 38.5-40 DESCR: CME 45C 38.5-40

OPERATOR: ZM

FILE: SPT CAL\_38.5\_40\_Log 5/24/2018 11:54:19 AM

Blow Number 9

#### Pile Properties

LE 43.70 ft AR 1.18 in^2 ΕM 30000 ksi SP 0.492 k/ft3 WS 16807.9 f/s EA/C 2.1 ksec/ft 2L/C 5.22 ms JC 37.50 ft LP

#### Quantity Results

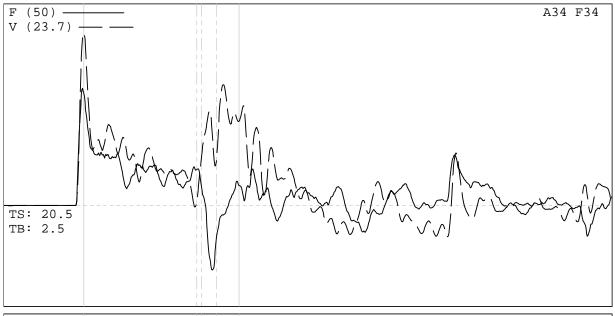
ETR 98.2 (%)
CSX 24.6 ksi
CSB 0.0 ksi
BPM 55.1 bpm
EMX 0.344 k-ft
DMX 0.69 in
SFR kips
MEX 820 µE
VMX 20.3 f/s

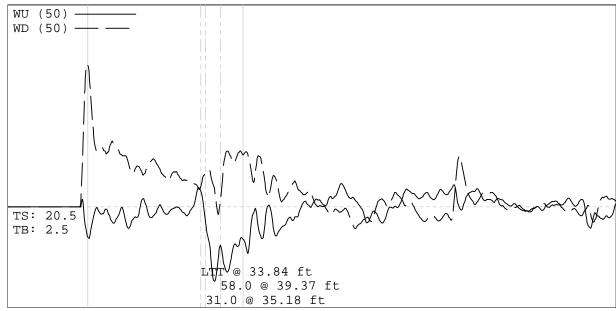
#### **Sensors**

F3: [AWJ 1] 216.53 (1) F4: [AWJ 2] 216.2 (1) A3: [K5418] 382 mv/5000g's (1) A4: [K0059] 317 mv/5000g's (1) CLIP: OK

#### **GASTON 2 SOLAR FACILITY**

SPT CAL 38.5-40





#### **Project Information**

PROJECT: GASTON 2 SOLAR FACILITY

PILE NAME: SPT CAL 38.5-40 DESCR: CME 45C 38.5-40

OPERATOR: ZM

FILE: SPT CAL\_38.5\_40\_Log 5/24/2018 11:54:19 AM

Blow Number 9

#### Pile Properties

LE 43.70 ft AR 1.18 in^2 ΕM 30000 ksi SP 0.492 k/ft3 WS 16807.9 f/s EA/C 2.1 ksec/ft 2L/C 5.22 ms JC 37.50 ft LP

#### Quantity Results

ETR 98.2 (%)
CSX 24.6 ksi
CSB 0.0 ksi
BPM 55.1 bpm
EMX 0.344 k-ft
DMX 0.69 in
SFR kips
MEX 820 µE
VMX 20.3 f/s

#### **Sensors**

F3: [AWJ 1] 216.53 (1) F4: [AWJ 2] 216.2 (1) A3: [K5418] 382 mv/5000g's (1) A4: [K0059] 317 mv/5000g's (1) CLIP: OK May 29, 2018 Terracon Project No. 73185061



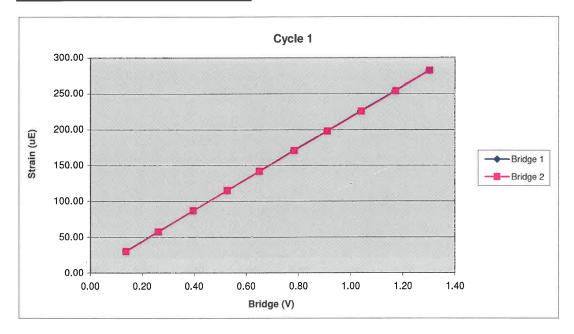
# **Exhibit A-2 PDA Equipment Calibrations**



267AWJ		Cycle 1		
Sample	Force (lb)	Strain (µE)	Bridge 1 (V)	Bridge 2 (V)
1	0.00	0.00	0.00	0.00
2	1034.65	29.80	0.14	0.14
3	1995.74	57.07	0.26	0.26
4	3021.54	86.56	0.40	0.40
5	4013.51	114.64	0.53	0.53
6	4945.09	141.59	0.65	0.65
7	5967.55	170.38	0.78	0.78
8	6935.72	197.78	0.91	0.91
9	7944.21	225.43	1.04	1.04
10	8935.59	253.69	1.17	1.17
11	9924.61	282.06	1.30	1.30

Bridge 1		Bridge 2	
Force Calibration (lb/V)	7636.82	Force Calibration (lb/V)	7622.38
Offset	-7.82	Offset	-11.05
Correlation	0.999996	Correlation	0.999997
Strain Calibration (µE/V)	216.43	Strain Calibration (µE/V)	216.02
Offset	0.60	Offset	0.51
Correlation	0.999996	Correlation	0.999993

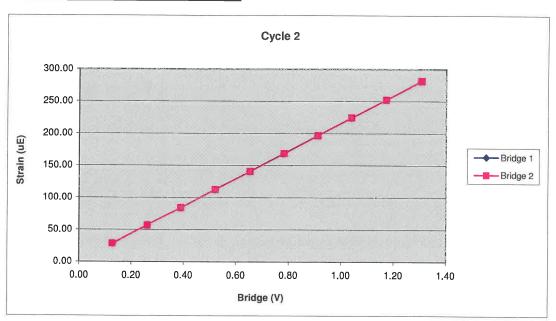
Force Strain Calibration	
EA (Kips)	35284.30
Offset	-29.01
Correlation	0.999989



267AWJ		Cycle 2		
Sample	Force (lb)	Strain (μE)	Bridge 1 (V)	Bridge 2 (V)
1	0.00	0.00	0.00	0.00
2	979.97	28.37	0.13	0.13
3	2002.82	57.00	0.26	0.26
4	2963.52	84.19	0.39	0.39
5	3965.12	112.62	0.52	0.52
6	4973.02	141.14	0.65	0.65
7	5964.01	169.18	0.78	0.78
8	6938.87	197.15	0.91	0.91
9	7932.41	224.93	1.04	1.04
10	8939.72	253.23	1.17	1.17
11	9962.18	281.86	1.30	1.31

Bridge 1		Bridge 2	
Force Calibration (lb/V)	7634.65	Force Calibration (lb/V)	7626.42
Offset	-9.30	Offset	-6.45
Correlation	0.999997	Correlation	0.999997
Strain Calibration (µE/V)	215.81	Strain Calibration (µE/V)	215.58
Offset	0.30	Offset	0.38
Correlation	0.999997	Correlation	0.999996

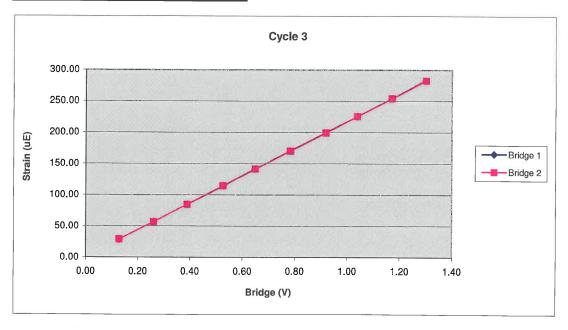
Force Strain Calibration	
EA (Kips)	35375.85
Offset	-19.88
Correlation	0.999997



267AWJ		Cycle 3		
Sample	Force (lb)	Strain (μE)	Bridge 1 (V)	Bridge 2 (V)
1	0.00	0.00	0.00	0.00
2	982.53	28.74	0.13	0.13
3	1993.97	56.89	0.26	0.26
4	2965.68	84.76	0.39	0.39
5	4014.49	114.61	0.53	0.53
6	4944.11	141.53	0.65	0.65
7	5962.04	170.43	0.78	0.78
8	6994.93	199.62	0.92	0.92
9	7922.38	225.92	1.04	1.04
10	8918.48	254.51	1.17	1.17
11	9909.26	283.02	1.30	1.30

Bridge 1		Bridge 2	
Force Calibration (lb/V)	7625.63	Force Calibration (lb/V)	7613.58
Offset	-4.78	Offset	-1.05
Correlation	0.999999	Correlation	0.999998
Strain Calibration (µE/V)	217.34	Strain Calibration (µE/V)	217.00
Offset	0.23	Offset	0.33
Correlation	0.999995	Correlation	0.999998

Force Strain Calibration	
EA (Kips)	35085.96
Offset	-12.70
Correlation	0.999996



Bridge Excitation (V) Shunt Resitor (ohm)

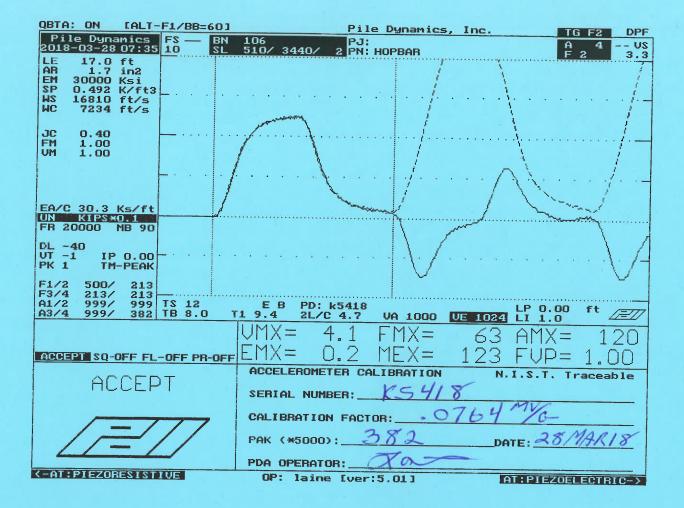
60.4k

Calibration Factors	267AWJ		
Bridge 1 (μΕ/V)	216.53	Bridge 2 (μΕ/V)	216.20
EA Factor (Kips)	35248.70	Area (in^2)	1.17

Calibrated by: Naval Br Calibrated Date:

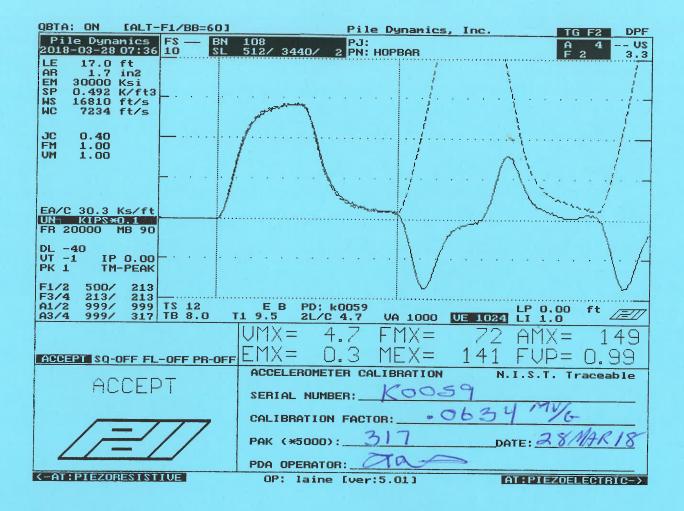
Pile Dynamics Inc 30725 Aurora Rd Solon, OH 44139

Traceable to N.I.S.T.



# **Smart Sensor**

Smart Chip Programmed By  $\overline{\mathcal{A}}, \overline{\mathcal{W}}$  on  $\underline{\mathcal{A}}, \underline{\mathcal{M}}, \underline{\mathcal{A}}, \underline{\mathcal{B}}$  CRC Value  $\underline{\mathcal{B}}, \underline{\mathcal{B}}, \underline{\mathcal{B}}$ 



**Smart Sensor** 

Smart Chip Programmed By  $\overline{\mathcal{A}.W.}$  on  $\underline{\mathcal{AR}NAR18}$  CRC Value  $\underline{A7E0}$ 

May 29, 2018 Terracon Project No. 73185061



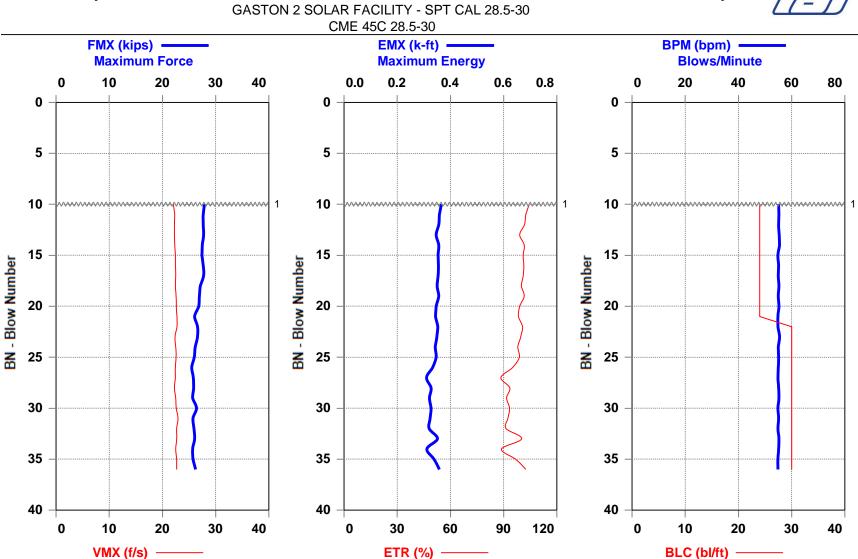
**Exhibit A-3** SPT Calibration Data Plots and Tables

Printed: 29-May-2018

GASTON 2 SOLAR FACILITY - SPT CAL 28 5-30



**Blow Count** 



**Energy Transfer Ratio - Rated** 

1 - Blows 1-9 = Seating Blows, Blow 10 = First Blow of N Value

**Maximum Velocity** 

GASTON 2 SOLAR FACILITY - SPT CAL 28.5-30 OP: ZM								CME 45C Date: 24-Ma	
AR:	1.18 in <sup>2</sup>								492 k/ft <sup>3</sup>
LE:	33.70 ft							EM: 30,	
	5,807.9 f/s								0.00
	Maximum Fo	rce				RPM·	Blows/Minu		
	Maximum Ve						Maximum D		nt
	Maximum En						Compression		
	Energy Trans		Rated			00/11	p. 200.0	• •	
BL#	Depth	BLC	FMX	VMX	EMX	ETR	BPM	DMX	CSX
	ft	bl/ft	kips	f/s	k-ft	(%)	bpm	in	ksi
10	29.04	24	28	22.0	0.366	104.5	55.1	0.93	23.6
11	29.08	24	28	22.3	0.359	102.5	55.2	0.85	23.4
12	29.13	24	28	22.3	0.356	101.7	55.1	0.76	23.4
13	29.17	24	28	22.2	0.347	99.1	55.3	0.67	23.5
14	29.21	24	28	22.3	0.356	101.6	55.4	0.74	23.3
15	29.25	24	27	22.4	0.353	101.0	54.8	0.68	23.3
16	29.29	24	28	22.5	0.355	101.4	55.1	0.70	23.4
17	29.33	24	28	22.5	0.354	101.0	55.0	0.64	23.5
18	29.38	24	27	22.4	0.350	100.0	55.2	0.62	23.0
19	29.42	24	27	22.6	0.356	101.6	55.0	0.64	22.8
20	29.46	24	27	22.6	0.347	99.1	55.2	0.64	22.7
21	29.50	24	26	22.7	0.344	98.4	54.8	0.60	22.1
22	29.53	30	27	22.7	0.352	100.6	54.9	0.63	22.5
23	29.57	30	27	22.4	0.349	99.6	55.4	0.60	22.5
24	29.60	30	26	22.5	0.343	98.0	55.0	0.56	22.2
25	29.63	30	26	22.6	0.346	98.8	55.1	0.59	22.0
26	29.67	30	26	22.4	0.333	95.1	55.0	0.53	21.6
27	29.70	30	26	22.4	0.310	88.5	54.8	0.48	21.9
28	29.73	30	26	22.3	0.327	93.5	55.1	0.50	21.9
29	29.77	30	26	22.5	0.321	91.6	55.2	0.49	21.8
30	29.80	30	26	22.6	0.327	93.3	54.8	0.49	22.4
31	29.83	30	26	22.9	0.323	92.3	55.1	0.49	21.8
32	29.87	30	26	22.6	0.320	91.3	54.8	0.48	21.9
33	29.90	30	26	22.7	0.351	100.2	55.2	0.63	22.1
34	29.93	30	26	22.5	0.311	88.8	55.1	0.47	21.7
35	29.97	30	26	22.6	0.338	96.7	54.8	0.52	21.8
36	30.00	30	26	22.7	0.359	102.5	54.9	0.69	22.2
		Average	27	22.5	0.343	97.9	55.1	0.62	22.5
		td. Dev.	1	0.2	0.015	4.4	0.2	0.11	0.7
		aximum	28	22.9	0.366	104.5	55.4	0.93	23.6
	IV	linimum	26	22.0	0.310	88.5	54.8	0.47	21.6
	Total number of blows analyzed: 27								

10-36 F3: [AWJ 1] 216.5 (1.00); F4: [AWJ 2] 216.2 (1.00); A3: [K5418] 382.0 (1.00); A4: [K0059] 317.0 (1.00)

### **BL#** Comments

10 Blows 1-9 = Seating Blows, Blow 10 = First Blow of N Value

Pile Dynamics, Inc. Case Method & iCAP® Results Page 2 PDIPLOT2 2017.2.58.3 - Printed 29-May-2018

GASTON 2 SOLAR FACILITY - SPT CAL 28.5-30 OP: ZM

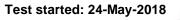
CME 45C 28.5-30 Date: 24-May-2018

Time Summary

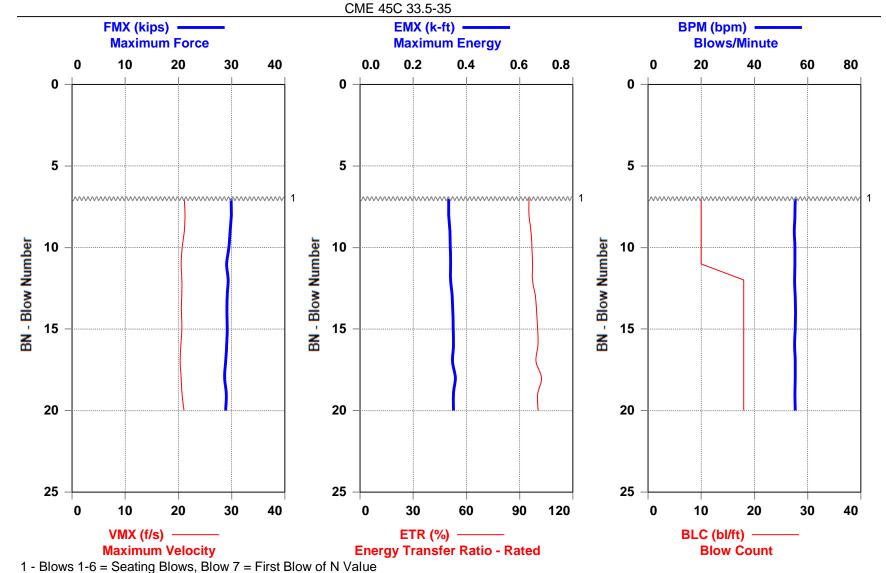
Drive 38 seconds 11:32 AM - 11:33 AM BN 1 - 36

Printed: 29-May-2018

GASTON 2 SOLAR FACILITY - SPT CAL 33.5-35







GASTON 2 SOLAR FACILITY - SPT CAL 33.5-35  OP: ZM  CME 45C 33.5  Date: 24-May-2									
AR:	1.18 in <sup>2</sup>								492 k/ft <sup>3</sup>
LE:	38.70 ft								000 ksi
	6,807.9 f/s								0.00
	Maximum F	orce				RPM·	Blows/Minut		3.00
	Maximum V						Maximum D		nt
	Maximum E						Compression		
		nsfer Ratio -	Rated			00/11	O 0111 p 1 0 0 0 1 0	• •	a, a, r, r a r r r
BL#	Depth	BLC	FMX	VMX	EMX	ETR	BPM	DMX	CSX
	· ft	bl/ft	kips	f/s	k-ft	(%)	bpm	in	ksi
7	34.10	10	30	21.1	0.333	95.3	55.3	1.21	25.3
8	34.20	10	30	21.2	0.333	95.3	55.2	1.21	25.4
9	34.30	10	30	21.1	0.337	96.4	55.0	1.23	25.2
10	34.40	10	29	20.7	0.339	97.0	55.2	1.20	25.0
11	34.50	10	29	20.5	0.341	97.3	55.2	1.22	24.6
12	34.56	18	29	20.6	0.340	97.2	55.1	1.02	24.8
13	34.61	18	29	20.6	0.346	98.9	55.3	0.95	24.7
14	34.67	18	29	20.5	0.349	99.6	55.4	0.91	24.7
15	34.72	18	29	20.7	0.350	100.1	55.3	0.88	24.7
16	34.78	18	29	20.5	0.351	100.3	55.1	0.85	24.6
17	34.83	18	29	20.3	0.347	99.2	55.3	0.77	24.5
18	34.89	18	29	20.5	0.358	102.3	55.3	0.82	24.3
19	34.94	18	29	20.7	0.351	100.2	55.2	0.76	24.6
20	35.00	18	29	21.0	0.351	100.4	55.3	0.73	24.5
		Average	29	20.7	0.345	98.5	55.2	0.98	24.8
	;	Std. Dev.	0	0.3	0.007	2.1	0.1	0.19	0.3
	N	/laximum	30	21.2	0.358	102.3	55.4	1.23	25.4
		Minimum	29	20.3	0.333	95.3	55.0	0.73	24.3
Total number of blows analyzed: 14									

7-20 F3: [AWJ 1] 216.5 (1.00); F4: [AWJ 2] 216.2 (1.00); A3: [K5418] 382.0 (1.00); A4: [K0059] 317.0 (1.00)

#### **BL# Comments**

7 Blows 1-6 = Seating Blows, Blow 7 = First Blow of N Value

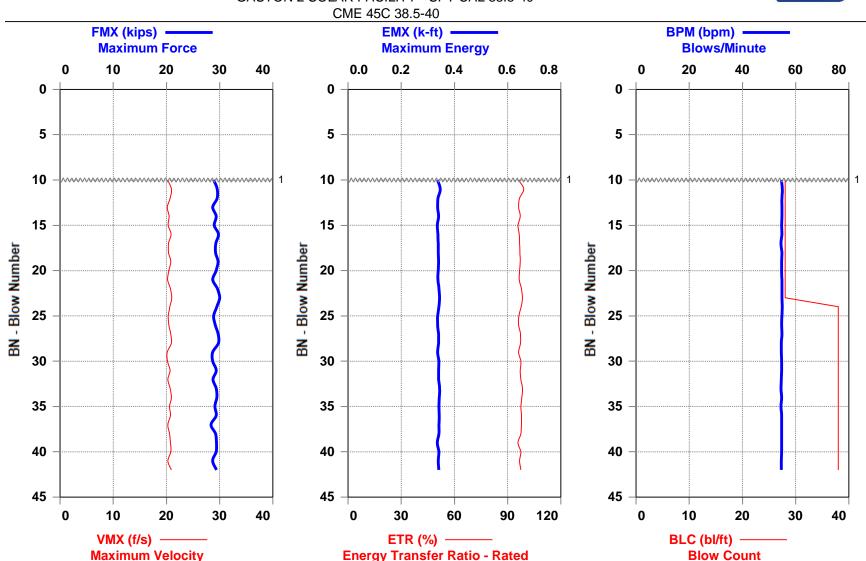
# Time Summary

Drive 20 seconds 11:48 AM - 11:48 AM BN 1 - 20

Printed: 29-May-2018

GASTON 2 SOLAR FACILITY - SPT CAL 38.5-40





1 - Blows 1-9 = Seating Blows, Blow 10 = First Blow of N Value

GASTON 2 SOLAR FACILITY - SPT CAL 38.5-40									
			CME 45C 38.5-40 Date: 24-May-2018						
OP: Z AR:	.ivi 1.18 in²								492 k/ft <sup>3</sup>
LE:	43.70 ft							SP: 0.4 EM: 30,0	
	43.70 ft 16,807.9 f/s								0.00 KSI
	Maximum F	-orce				RDM: I	Blows/Minut		7.00
	Maximum \		isplacemen	nt					
	Maximum E		n Stress Ma						
		nsfer Ratio -	Rated				p. 000.0	• • •	
BL#	Depth	BLC	FMX	VMX	EMX	ETR	BPM	DMX	CSX
	ft	bl/ft	kips	f/s	k-ft	(%)	bpm	in	ksi
10	37.54	28	29	20.2	0.336	96.1	54.6	0.58	24.5
11	37.57	28	30	20.9	0.347	99.1	55.0	0.59	25.0
12	37.61	28	30	20.7	0.338	96.7	54.8	0.57	25.0
13	37.64	28	29	20.2	0.337	96.1	54.8	0.56	24.3
14	37.68	28	29	20.5	0.340	97.1	54.8	0.56	24.9
15	37.71	28	29	20.3	0.335	95.9	54.7	0.56	24.6
16	37.75	28	30	20.9	0.338	96.5	54.8	0.55	25.2
17	37.79	28	29	20.4	0.339	96.7	54.5	0.56	24.8
18	37.82	28	29	20.4	0.339	96.9	54.8	0.56	24.8
19	37.86	28	30	20.8	0.340	97.1	54.7	0.55	25.2
20	37.89	28	29	20.4	0.339	96.7	54.7	0.56	24.8
21	37.93	28	29	20.2	0.338	96.5	54.8	0.55	24.3
22	37.96	28	30	20.7	0.342	97.7	54.9	0.56	25.1
23 24	38.00	28 38	30	21.0	0.344 0.342	98.3	54.8	0.56	25.4
24 25	38.03 38.05	36 38	29 29	20.6 20.3	0.342	97.7 96.3	55.0 54.8	0.56 0.55	25.0 24.5
26	38.08	38	29 29	20.3	0.337	96.3 96.2	54.6 54.7	0.53	24.5 24.7
27	38.11	38	30	20.4	0.340	97.1	54.7 54.8	0.54	24.7 25.2
28	38.13	38	30	20.8	0.340	97.1	54.6	0.53	25.2 25.2
29	38.16	38	29	20.3	0.336	96.1	54.6	0.53	24.3
30	38.18	38	29	20.2	0.341	97.5	54.7	0.53	24.3
31	38.21	38	29	20.7	0.340	97.1	54.7	0.52	24.9
32	38.24	38	29	20.3	0.341	97.4	54.6	0.51	24.4
33	38.26	38	29	20.7	0.344	98.4	54.5	0.51	24.9
34	38.29	38	29	20.9	0.343	98.0	54.7	0.51	25.0
35	38.32	38	29	20.6	0.341	97.5	54.5	0.50	24.7
36	38.34	38	29	20.8	0.342	97.8	54.7	0.50	24.9
37	38.37	38	28	20.2	0.342	97.7	54.7	0.50	24.0
38	38.39	38	29	20.6	0.341	97.5	54.7	0.49	24.8
39	38.42	38	29	20.7	0.335	95.8	54.7	0.48	24.9
40	38.45	38	29	20.8	0.341	97.5	54.7	0.49	24.9
41	38.47	38	29	20.3	0.339	96.7	54.6	0.48	24.3
42	38.50	38	29	20.9	0.341	97.5	54.6	0.49	24.9
		Average	29	20.6	0.340	97.1	54.7	0.53	24.8
		Std. Dev.	0	0.3	0.003	0.8	0.1	0.03	0.3
		Maximum	30	21.0	0.347	99.1	55.0	0.59	25.4
		Minimum	28 Total n	20.2	0.335	95.8	54.5	0.48	24.0
			i otal n	umber of t	olows analyz	zeu: 33			

10-42 F3: [AWJ 1] 216.5 (1.00); F4: [AWJ 2] 216.2 (1.00); A3: [K5418] 382.0 (1.00); A4: [K0059] 317.0 (1.00)

# GASTON 2 SOLAR FACILITY - SPT CAL 38.5-40 OP: ZM

CME 45C 38.5-40 Date: 24-May-2018

# BL# Comments

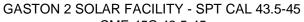
10 Blows 1-9 = Seating Blows, Blow 10 = First Blow of N Value

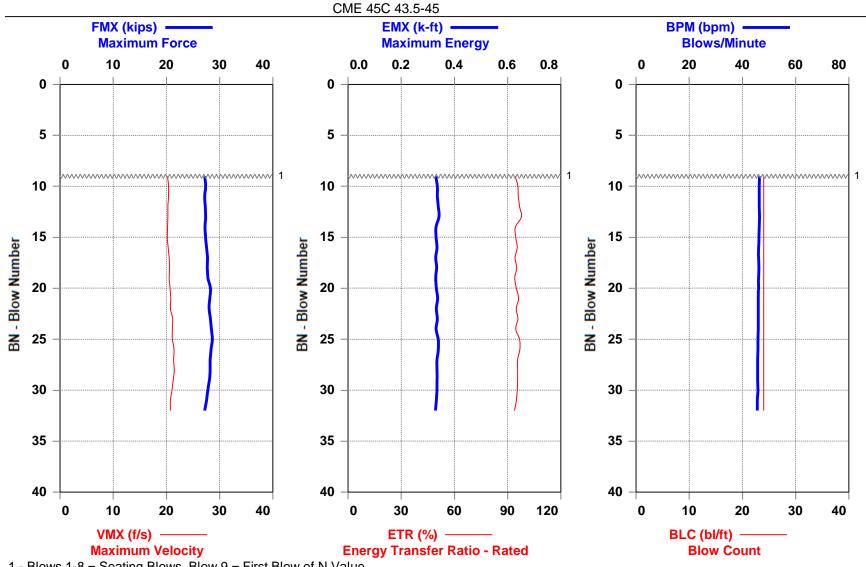
# Time Summary

Drive 44 seconds 11:54 AM - 11:54 AM BN 1 - 42

**Printed: 29-May-2018** 







<sup>1 -</sup> Blows 1-8 = Seating Blows, Blow 9 = First Blow of N Value

GAST OP: Z	ON 2 SOLAF	R FACILITY			CME 45C Date: 24-Ma						
AR:	1.18 in <sup>2</sup>								492 k/ft <sup>3</sup>		
LE:	48.70 ft							EM: 30,			
	6,807.9 f/s			000 KSi							
	Maximum Fo	Blows/Minut		7.00							
	Maximum Ve			Maximum D		nt					
	Maximum Er			Compression							
	ETR: Energy Transfer Ratio - Rated										
BL#	Depth	BLC	FMX	VMX	EMX	ETR	BPM	DMX	CSX		
	· ft	bl/ft	kips	f/s	k-ft	(%)	bpm	in	ksi		
9	44.04	24	27	20.2	0.330	94.2	46.4	0.67	23.0		
10	44.08	24	27	20.4	0.335	95.8	46.3	0.65	23.2		
11	44.13	24	27	20.4	0.336	95.9	46.3	0.65	23.0		
12	44.17	24	27	20.3	0.339	96.8	46.3	0.69	23.1		
13	44.21	24	27	20.3	0.342	97.7	46.4	0.75	23.2		
14	44.25	24	27	20.2	0.330	94.4	46.3	0.64	23.1		
15	44.29	24	27	20.1	0.331	94.6	46.2	0.63	23.2		
16	44.33	24	28	20.3	0.334	95.4	46.1	0.65	23.3		
17	44.38	24	28	20.5	0.330	94.2	46.0	0.63	23.5		
18	44.42	24	28	20.6	0.333	95.0	46.2	0.62	23.5		
19	44.46	24	28	20.5	0.329	94.0	46.0	0.62	23.6		
20	44.50	24	28	20.7	0.332	94.9	46.0	0.62	23.9		
21	44.54	24	28	20.8	0.337	96.2	45.9	0.62	23.9		
22	44.58	24	28	20.7	0.332	94.8	45.9	0.62	23.7		
23	44.63	24	28	21.2	0.335	95.7	45.9	0.62	24.0		
24	44.67	24	28	21.1	0.331	94.5	45.9	0.61	24.1		
25	44.71	24	29	21.1	0.339	96.7	45.8	0.60	24.3		
26	44.75	24	28	21.4	0.339	96.9	45.8	0.78	24.0		
27	44.79	24	28	21.4	0.334	95.5	45.7	0.61	23.9		
28	44.83	24	28	21.5	0.335	95.7	45.7	0.60	23.9		
29	44.88	24	28	21.3	0.335	95.6	45.7	0.59	23.8		
30	44.92	24	28	21.0	0.334	95.4	45.8	0.63	23.5		
31	44.96	24	28	20.8	0.332	94.9	45.6	0.60	23.3		
32	45.00	24	27	20.7	0.329	93.9	45.6	0.57	23.0		
		Average	28	20.7	0.334	95.4	46.0	0.64	23.5		
		Std. Dev.	0	0.4	0.003	1.0	0.3	0.05	0.4		
		laximum	29	21.5	0.342	97.7	46.4	0.78	24.3		
	N	/linimum	27	20.1	0.329	93.9	45.6	0.57	23.0		
	Total number of blows analyzed: 24										

9-32 F3: [AWJ 1] 216.5 (1.00); F4: [AWJ 2] 216.2 (1.00); A3: [K5418] 382.0 (1.00); A4: [K0059] 317.0 (1.00)

#### **BL# Comments**

9 Blows 1-8 = Seating Blows, Blow 9 = First Blow of N Value

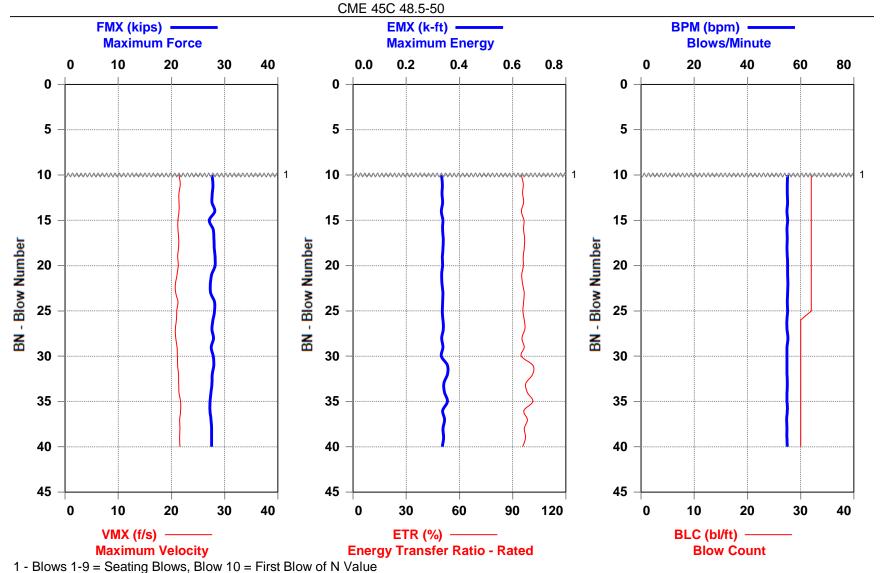
# Time Summary

Drive 40 seconds 12:01 PM - 12:02 PM BN 1 - 32

Printed: 29-May-2018



GASTON 2 SOLAR FACILITY - SPT CAL 48.5-50



GASTO OP: ZM	ON 2 SOLAR	FACILITY		CME 45C 48.5-50 Date: 24-May-2018							
AR:	1.18 in²								492 k/ft <sup>3</sup>		
LE:	53.70 ft		EM: 30,000 ksi								
WS: 16			000 KSI								
	Maximum Fo	Dlave /Misus		7.00							
	viaximum Fo Viaximum Ve				BPM: Blows/Minute DMX: Maximum Displacement						
	viaximum ve Viaximum En						on Stress Ma				
	Energy Trans		Patad			CSA.	Compressio	JII SUESS IVI	axiiiiuiii		
BL#		BLC	FMX	VMX	EMX	ETR	BPM	DMX	CSX		
DL#	Depth	bl/ft		f/s	⊏ivi∧ k-ft	(%)		in	ksi		
10	ft 49.03	32	kips 28	21.4	0.332	94.8	bpm 55.1	0.63	23.4		
11		32 32	28	21.4	0.332	94.6 96.0	55.1 55.0	0.63	23.4		
	49.06 49.09	32 32	28	21.7	0.334			0.63	23.5		
12						95.5	55.0				
13	49.13	32	28	21.4	0.337	96.2	55.1	0.63	23.4		
14	49.16	32	28	21.4	0.332	94.9	54.8	0.62	23.8		
15	49.19	32	27	21.2	0.337	96.4	55.1	0.63	23.0		
16	49.22	32	28	21.2	0.336	96.1	54.8	0.63	23.6		
17	49.25	32	28	21.4	0.339	96.8	55.0	0.64	23.7		
18	49.28	32	28	21.3	0.338	96.6	54.9	0.63	23.8		
19	49.31	32	28	21.2	0.336	96.0	55.0	0.63	23.9		
20	49.34	32	28	21.3	0.336	96.0	55.1	0.63	23.9		
21	49.38	32	28	21.0	0.333	95.1	55.1	0.63	23.4		
22	49.41	32	27	20.8	0.334	95.5	55.2	0.64	23.2		
23	49.44	32	27	20.8	0.338	96.4	55.1	0.65	23.2		
24	49.47	32	28	21.2	0.337	96.2	55.0	0.65	23.8		
25	49.50	32	28	21.0	0.335	95.8	55.1	0.64	23.8		
26	49.53	30	28	20.9	0.338	96.5	54.8	0.63	23.6		
27	49.57	30	28	20.7	0.339	96.9	54.9	0.64	23.4		
28	49.60	30	28	20.8	0.333	95.2	55.2	0.61	23.6		
29	49.63	30	27	21.0	0.338	96.5	54.9	0.65	23.3		
30	49.67	30	28	21.1	0.332	94.8	54.9	0.58	23.6		
31	49.70	30	28	21.1	0.355	101.4	54.9	0.88	23.7		
32	49.73	30	28	21.3	0.354	101.2	54.9	0.82	23.4		
33	49.77	30	28	21.4	0.341	97.5	55.0	0.68	23.4		
34	49.80	30	27	21.4	0.344	98.3	54.9	0.71	23.2		
35	49.83	30	27	21.7	0.355	101.4	54.9	0.83	23.1		
36	49.87	30	27	21.7	0.337	96.4	55.0	0.59	23.1		
37	49.90	30	27	21.6	0.344	98.2	54.7	0.69	23.3		
38	49.93	30	28	21.6	0.338	96.6	54.8	0.59	23.3		
39	49.97	30	28	21.5	0.340	97.2	54.8	0.58	23.4		
40	50.00	30	28	21.6	0.335	95.7	55.0	0.58	23.4		
		Average	28	21.3	0.339	96.7	55.0	0.65	23.5		
	Std. Dev.			0.3	0.006	1.7	0.1	0.07	0.3		
		aximum	0 28	21.7	0.355	101.4	55.2	0.88	23.9		
		linimum	27	20.7	0.332	94.8	54.7	0.58	23.0		
				-	olows analy						

10-40 F3: [AWJ 1] 216.5 (1.00); F4: [AWJ 2] 216.2 (1.00); A3: [K5418] 382.0 (1.00); A4: [K0059] 317.0 (1.00)

# **BL#** Comments

10 Blows 1-9 = Seating Blows, Blow 10 = First Blow of N Value

Pile Dynamics, Inc. Case Method & iCAP® Results

Page 2 PDIPLOT2 2017.2.58.3 - Printed 29-May-2018

GASTON 2 SOLAR FACILITY - SPT CAL 48.5-50 OP: ZM

CME 45C 48.5-50 Date: 24-May-2018

Time Summary

Drive 42 seconds 12:10 PM - 12:11 PM BN 1 - 40

Drill Rig SPT Hammer Energy Calibration Report CME-45C (SN 406484) ■ Columbia, SC

May 29, 2018 Terracon Project No. 73185061



# Exhibit A-4 Field Log

# lerracon

	SP	T HAM			IBRAT HEET	ΓΙΟΝ			CLIENT RE	P:	lar.					
PRO.			DATE	DATE: 5/24												
PROJECT NAME: Gaston. Solar Facility PROJECT NO .: 7219506							TFRRA									
PROJECT NO.: 73/95061  BORING NO.: 8-18					_											
	CLIENT	:	W I	9		– TERR	PDA MODEL/SN:  TERRACON RIG # DR#: 543									
	DB	ILL RIG	DATA								Λ					
Typ	pe/Transport					SPT HAMMER DATA  Type:										
	/lanufacturer		-451		-	Manufacturer:										
	Model No.:	CME-	45C			Lifting Mechanism: Claim us/ Hyd							ح ما انح			
	Serial No:	406	454			_		Model No.			<u> </u>		<u> </u>			
	Year Built:	4-	18			_		Serial No	The state of the s							
N	/lodifications:	NI	A			_		ner Weight								
Maii	nt. Schedule:	As,	Need	<u>ed</u>		_ Ha	mmer C	perator(s)	Salare	erc	znd	Bria	en			
					PDA	INPUT DAT	Α									
	Operator:	OP	Z	M		Ela	stic Mo	dulus (ksi):	: EM 3000							
Project N	No./Location:	PJ			Cosse	_ ≤∽ Specifi	c Weigl	nt (kips/ft <sup>3</sup> ):	SP							
Rig	Model & SN:	PN	CME.	-45C	-	_ Wa	ws	rs 16808								
Hammer Type, LM, Rods: PD Live, Cyc				, cha	in, Au	Increment Length (ft): LI 0.5										
Drill Rod Area (in <sup>2</sup> ): AR 1.18					Sampling Freq. (kHz): FR 50											
TRANSDUCER INFORMATION						NOTES: Instrumentation to Bottom of Rod Length 35.85+ 2.85										
Gage	5	SN		Cali	bration	Inches Feet										
F1/F3:	AWJ	-\	_	26	-53								-			
F2/F4:	AUT	- 2	_		.20	SPLIT SPOON SAMPLER LENGTH										
A1/A3:	K5418	<del></del>	_	38	3	Guage to Botton	n of Cal	. Rod leng	jth	28	3.7					
A2/A4:	KC05	9		31	7	LE is Measure		the Cente	r of the S	Strain Gu	ages to	the bott	tom of			
				SP	T TEST	Split Spoon San		N								
2		Stick Up		th (ft)				PDA Blows		SPT Blov		Blows	NS			
Start Time	Soil	Length (ft)		End	<sup>1</sup> LE (ft)	Rods & Leng	gths	Start	End	1st 6"	2nd 6"	3rd 6"	41170			
						2ft (5ft)	10ft						N			
10:50		32"	23.5	25	28.7	Sea 2ft Sft	10ft						1			
11120		41.5	24,5	26	30.7	1-2ft 5,5ft	10ft	1	100	5	5	8	13			
11:32	Sand	36"	28.5	30	33.7	2ft 5ft	10ft	1	37	9	12	15	27			
11:48	White Sand	36"	33.5	35	38.7	2ft 7 5th	10ft	ı	21	6	5	a	14			
11:53	Orange San	37"	38.5	40	437	2ft 8x 5ft	10ft	1	TAS	9	14	19	33			
12:61	White Sand	7::	43.5	45	48.7.	2ft 4x(5ft)	10ft	1	33	8	12	12	24			
12:10	Sand	35"	48.5	50	53.7	2ft 10x Sty	10ft	,	41	9	16	15	31			
						2ft 5ft	10ft									
Individual pair	s of F or V s	ignals versus	s time sha	all be ver	y similar for	good quality data.					1					
If you see For	ce goes neg	ative before	2L/C afte	r impact,	drill rod joir	nts should be caref	ully tigh	tened for g	ood quali	ty data						
DICTUDE AUI	MBEDS AND	D INFO: Tak	o Photo	of East	Digo Dori	ng Locations at th	0 614-									
Driling Metho						ig Locations at th	ie oite									

10:30

12:30

3 hr

ARRIVAL TIME: DEPART TIME:

TOTAL TRAVEL: